

FrameCad System

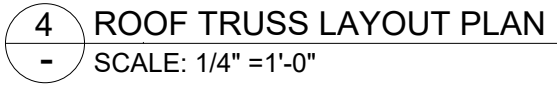
FRAMECAD			
Truss name	Weight(lbs)	Quantity	Total weight(lbs.)
E200	17	12	204
E201	16	2	32
HN1	37.6	2	75.2
HN2	38.1	9	342.9
HN3	164	7	1148
G1	294	1	294
HG1	16.1	2	32.2
HG2	134.4	1	134.4
Total Roof Weight (lbs.)			2262.7

Scotsteel System

SCOTSTEEL(2" HAT)			
Truss name	Weight(lbs)	Quantity	Total weight(lbs.)
E200	9.96	12	119.52
E201	9.22	2	18.44
HN1	28.63	2	57.26
HN2	28.63	9	257.67
HN3	91.83	7	642.81
GI1	294	1	251.35
HG1	28.63	2	57.26
HG2	134.4	1	91.83
Total Roof Weight (lbs.)			1496.14

FRAMECAD

Truss name	Weight(lbs)	Quantity	Total weight(lbs.)
E200	17	12	204
E201	16	2	32
HN1	37.6	2	75.2
HN2	38.1	9	342.9
HN3	164	7	1148
G1	294	1	294
HG1	16.1	2	32.2
HG2	134.4	1	134.4
Total Roof Weight (lbs.)			2262.7



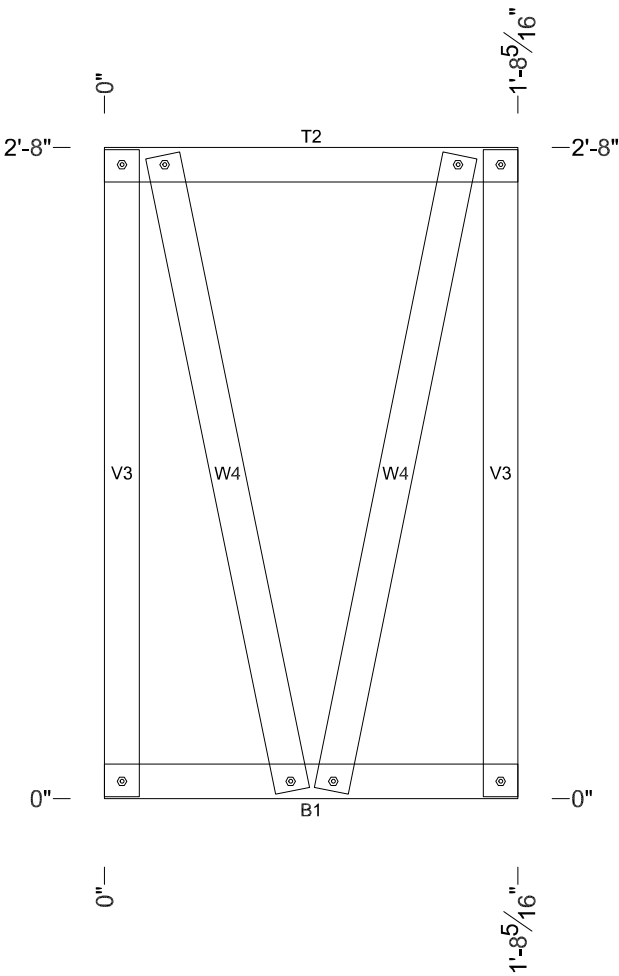
A. REF. TO ENGINEERED SHOP DRAWINGS FOR FASTENER & TRUSS DETAILS.

S-04

B1	362S162-43-50	6	1'-8 5/16"	T2	362S162-43-50	6	1'-8 5/16"	V3	362S162-43-50	12	2'-7 13/16"	W4	362S162-43-50	12	2'-7 7/8"
Assembly Weight				17lbs	Working Sheet: Floor.1						FRAMECAD 10g-16mm Flathead				96
											FRAMECAD 10g-19mm XDrive				96

Powered by FRAMECAD Structure ®

Minimum number of fasteners required is 4 per joint



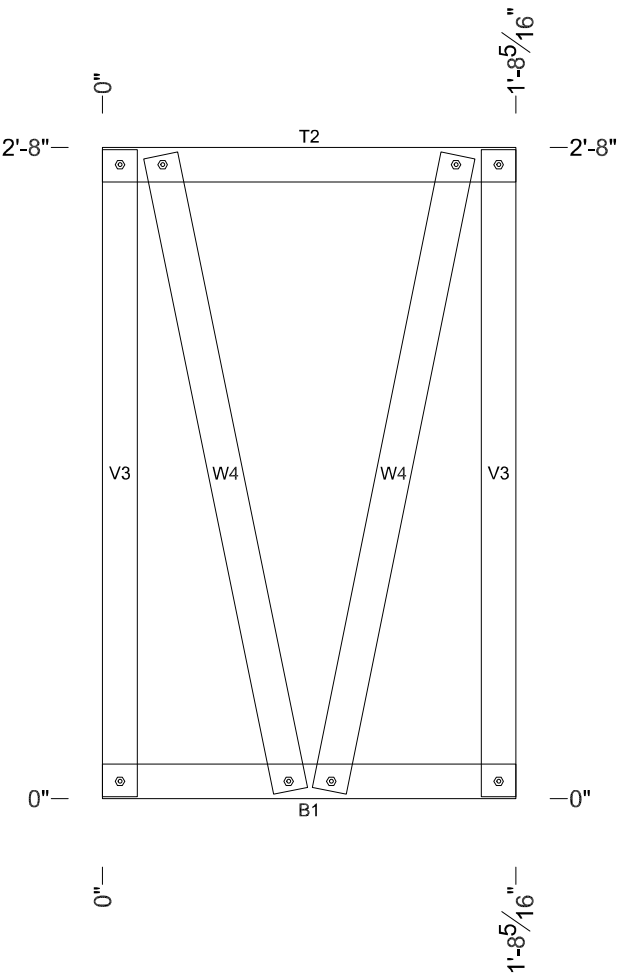
Beam Type WJ362043-50 Mark as E200 Quantity Required = 6

System Name:	FRAMECAD_FT_i	Joist Spacing:	1'-4"	Design Code:	AISI S100-12 LRFD	Loading Code:	IBC 2018 LRFD
Top Chord Restraints:	11 13/16"	Design Dead Load (psf):	10.0	Design Live Load (psf):	40.0	Design Ceiling Load (psf):	5.0
Design Snow Load (psf):	0.0	Design Point Load (Kip):	0.40	Envelope:	2'-8"h x 1'-8 5/16"w		

B1	362S162-43-50	6	1'-8 5/16"	T2	362S162-43-50	6	1'-8 5/16"	V3	362S162-43-50	12	2'-7 13/16"	W4	362S162-43-50	12	2'-7 7/8"
Assembly Weight				17lbs	Working Sheet: Floor.1						FRAMECAD 10g-16mm Flathead				96
											FRAMECAD 10g-19mm XDrive				96

Powered by FRAMECAD Structure ®

Minimum number of fasteners required is 4 per joint



Beam Type WJ362043-50 Mark as E200 Quantity Required = 6

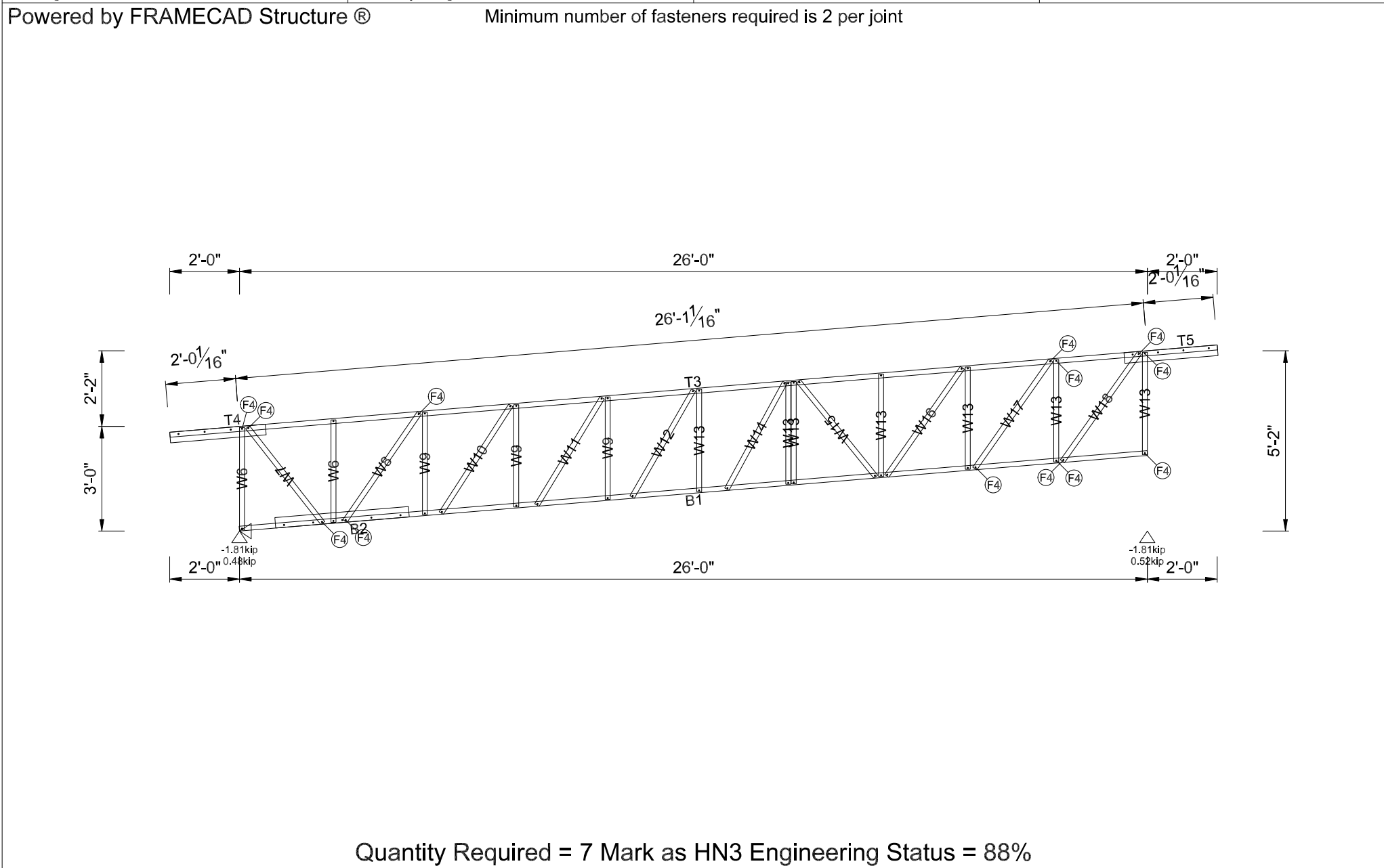
System Name:	FRAMECAD_FT_i	Joist Spacing:	1'-4"	Design Code:	AISI S100-12 LRFD	Loading Code:	IBC 2018 LRFD
Top Chord Restraints:	11 13/16"	Design Dead Load (psf):	10.0	Design Live Load (psf):	40.0	Design Ceiling Load (psf):	5.0
Design Snow Load (psf):	0.0	Design Point Load (Kip):	0.40	Envelope:	2'-8"h x 1'-8 5/16"w		

Powered by FRAMECAD Structure® Minimum number of fasteners required is 4 per joint



_nextgenstructure Dwg: Lighthouse 4B RFI View 9 of 24 Client: Lighthouse Job No:

B1	362S162-43-50	7	26'-1 3/16"	B2	362S162-43-50	7	3'-10 1/16"	T3	362S162-43-50	7	30'-1 1/4"	T4	362S162-43-50	7	2'-8 15/16"
T5	362S162-43-50	7	2'-8 3/16"	W6	362S162-43-50	14	2'-11 1/4"	W7	362S162-43-50	7	3'-5 7/8"	W8	362S162-43-50	7	3'-9 1/8"
W9	362S162-43-50	21	2'-11 3/16"	W10	362S162-43-50	7	3'-8 1/16"	W11	362S162-43-50	7	3'-7 3/16"	W12	362S162-43-50	7	3'-6 7/16"
W13	362S162-43-50	49	2'-11 3/16"	W14	362S162-43-50	7	3'-5 3/4"	W15	362S162-43-50	7	3'-6 3/16"	W16	362S162-43-50	7	3'-9 3/8"
W17	362S162-43-50	7	3'-9 1/2"	W18	362S162-43-50	7	3'-9 5/8"	FRAMECAD 10g-16mm Flathead	182			FRAMECAD 10g-19mm XDrive	798		
Working Sheet: Truss.2				Assembly Weight				164.0lbs							



Quantity Required = 7 Mark as HN3 Engineering Status = 88%

System Name:	FRAMECAD_FT_i	Roof Type:	SHEET	Truss Model:	M11 362 LE System	Wind Speed:	W114
Truss Pitch:	4.764	Truss Spacing:	2'-0"	Design Code:	AISI S100-12 LRFD	Loading Code:	IBC 2018 LRFD
Top Chord Live Load (psf):	20.0	Top Chord Dead Load (psf):	10.0	Bottom Chord Dead Load (psf):	5.0	Top Chord Snow Load (psf):	25.0
Envelope:	5'-4"h x 30'-5/16"w						

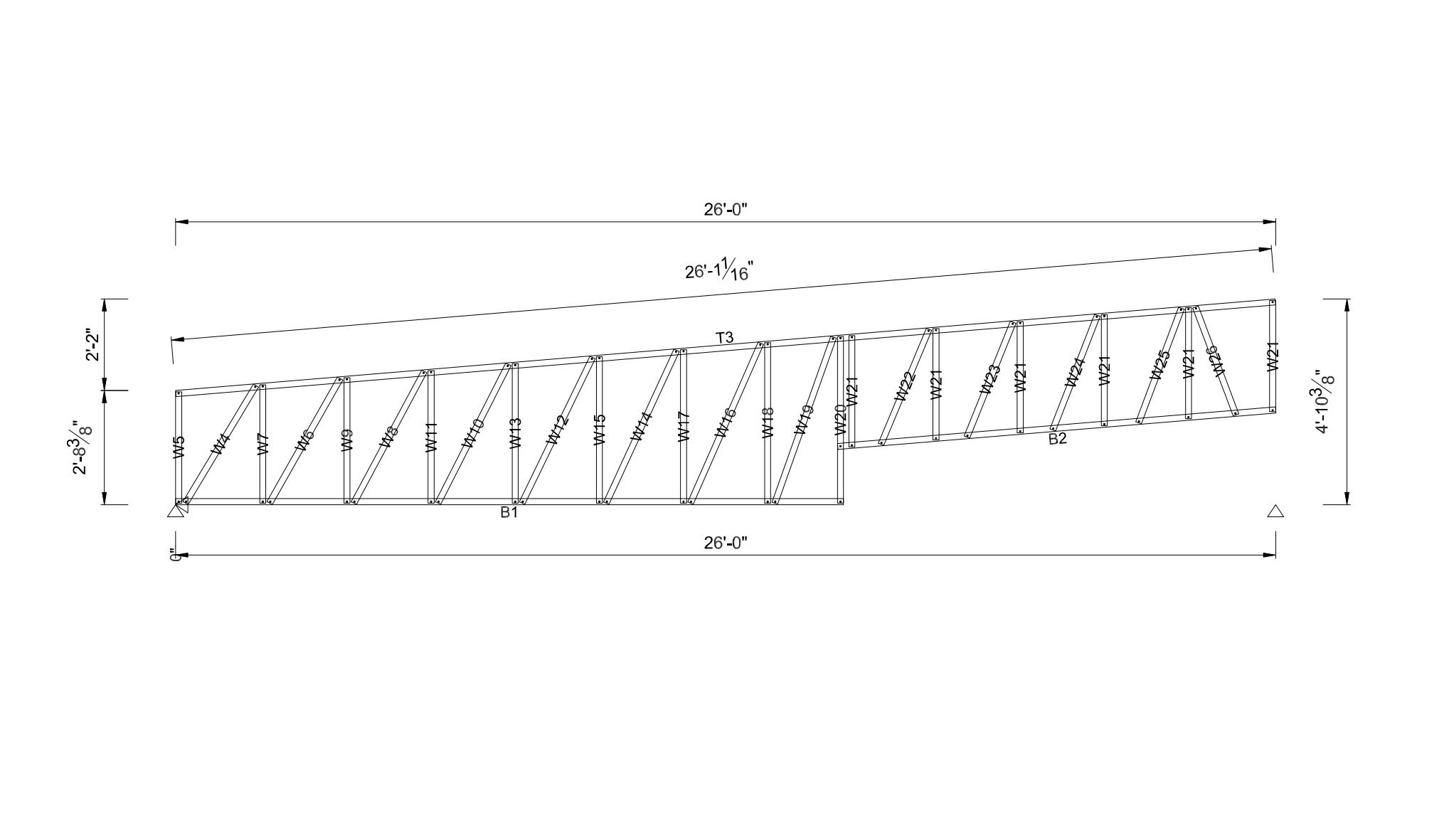
Powered by FRAMECAD Structure® Minimum number of fasteners required is 2 per joint



System Name:	FRAMECAD_FT_i	Roof Type:	SHEET	Truss Model:	M11 362 LE System	Wind Speed:	W114
Truss Pitch:	4.7636	Truss Spacing:	1'-4"	Design Code:	AISI S100-12 LRFD	Loading Code:	IBC 2018 LRFD
Top Chord Live Load (psf):	20.0	Top Chord Dead Load (psf):	10.0	Bottom Chord Dead Load (psf):	5.0	Top Chord Snow Load (psf):	25.0
Number of on-coming Trusses:	11	Envelope:	4'-10 3/8"h x 26'-1/8"w				

B1	362S162-33-50	1	15'-9 7/16"	B2	362S162-43-50	1	10'-4 11/16"	T3	362S162-33-50	1	26'-15/16"	W4	362S162-33-50	1	3'-2 7/8"
W5	362S162-33-50	1	2'-7 5/8"	W6	362S162-33-50	1	3'-4 9/16"	W7	362S162-33-50	1	2'-9 5/8"	W8	362S162-33-50	1	3'-6 5/16"
W9	362S162-33-50	1	2'-11 9/16"	W10	362S162-33-50	1	3'-8 1/8"	W11	362S162-33-50	1	3'-1 9/16"	W12	362S162-33-50	1	3'-9 7/8"
W13	362S162-33-50	1	3'-3 9/16"	W14	362S162-33-50	1	3'-11 11/16"	W15	362S162-33-50	1	3'-5 9/16"	W16	362S162-33-50	1	4'-1 1/2"
W17	362S162-33-50	1	3'-7 9/16"	W18	362S162-33-50	1	3'-9 9/16"	W19	362S162-33-50	1	4'-1 15/16"	W20	362S162-33-50	1	3'-11 1/4"
W21	362S162-33-50	6	2'-7 1/2"	W22	362S162-33-50	1	2'-11 5/16"	W23	362S162-33-50	1	2'-11 1/16"	W24	362S162-33-50	1	2'-10 7/8"
W25	362S162-33-50	1	2'-10 11/16"	W26	362S162-33-50	1	2'-8 5/8"	FRAMECAD 10g-19mm XDrive	114			Working Sheet: Truss HG2			
Assembly Weight			134.4lbs												

Powered by FRAMECAD Structure [®]
 Minimum number of fasteners required is 2 per joint

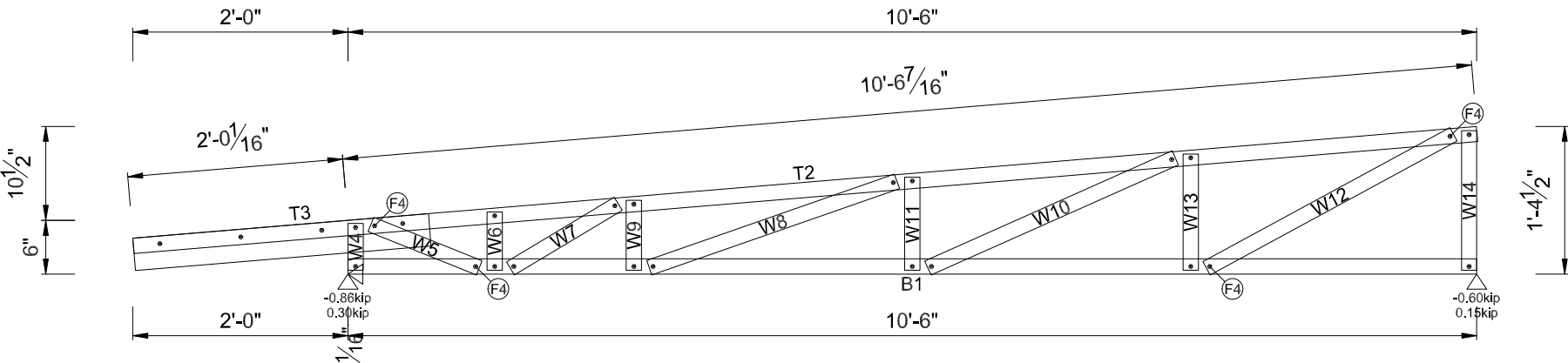


Quantity Required = 1 Mark as HG2 Engineering Status = Non Structural

System Name:	FRAMECAD_FT_i	Roof Type:	SHEET	Truss Model:	M11 362 LE System	Wind Speed:	W114
Truss Pitch:	4.764	Truss Spacing:	2'-0"	Envelope:	4'-10 3/8"h x 26'-1/8"w		

B1	362S162-33-50	2	10'-6"	T2	362S162-33-50	2	12'-6 1/2"	T3	362S162-33-50	2	2'-9 1/8"	W4	362S162-33-50	2	5 1/4"
W5	362S162-33-50	2	1'-1"	W6	362S162-33-50	2	6 1/2"	W7	362S162-33-50	2	1'-2"	W8	362S162-33-50	2	2'-5 1/4"
W9	362S162-33-50	2	7 13/16"	W10	362S162-33-50	2	2'-6 1/4"	W11	362S162-33-50	2	10 7/16"	W12	362S162-33-50	2	2'-7 3/8"
W13	362S162-33-50	2	1'-1"	W14	362S162-33-50	2	1'-3 9/16"	FRAMECAD 10g-16mm Flathead6				FRAMECAD 10g-19mm XDrive104			
Working Sheet: Truss.1				Assembly Weight				37.6lbs							

Powered by FRAMECAD Structure ® Minimum number of fasteners required is 2 per joint

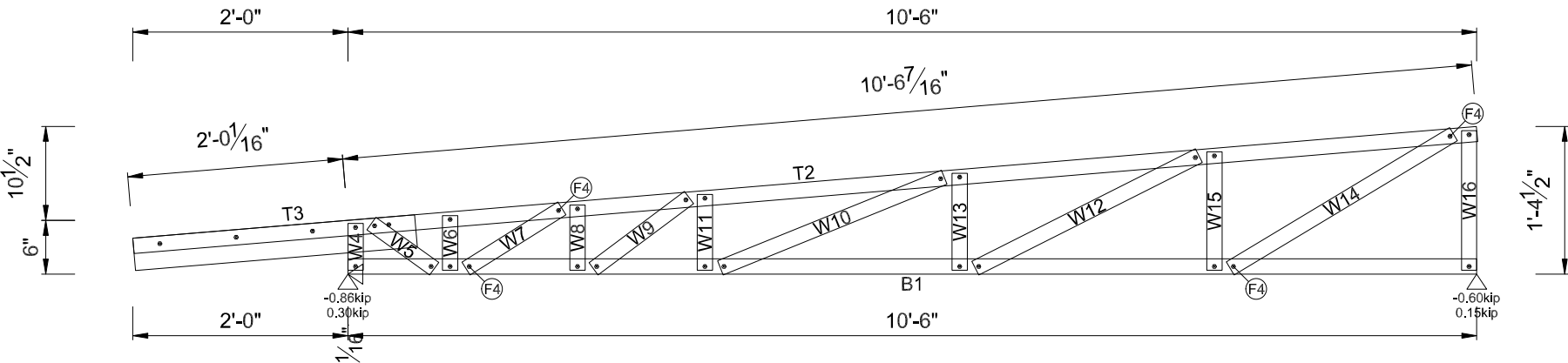


Quantity Required = 2 Mark as HN1 Engineering Status = 98%

System Name:	FRAMECAD_FT_i	Roof Type:	SHEET	Truss Model:	M11 362 LE System	Wind Speed:	W114
Truss Pitch:	4.764	Truss Spacing:	2'-0"	Design Code:	AISI S100-12 LRFD	Loading Code:	IBC 2018 LRFD
Top Chord Live Load (psf):	20.0	Top Chord Dead Load (psf):	10.0	Bottom Chord Dead Load (psf):	5.0	Top Chord Snow Load (psf):	25.0
Envelope:	1'-4 5/8"h x 12'-6 1/8"w						

B1	362S162-33-50	9	10'-6"	T2	362S162-33-50	9	12'-6 1/2"	T3	362S162-33-50	9	2'-7 9/16"	W4	362S162-33-50	9	5 1/4"
W5	362S162-33-50	9	8 5/8"	W6	362S162-33-50	9	6 1/8"	W7	362S162-33-50	9	1'-5/8"	W8	362S162-33-50	9	7 5/16"
W9	362S162-33-50	9	1'-1 5/16"	W10	362S162-33-50	9	2'-2 15/16"	W11	362S162-33-50	9	8 1/2"	W12	362S162-33-50	9	2'-3 15/16"
W13	362S162-33-50	9	10 7/8"	W14	362S162-33-50	9	2'-5 1/16"	W15	362S162-33-50	9	1'-1 1/4"	W16	362S162-33-50	9	1'-3 9/16"
FRAMECAD 10g-16mm Flathead				FRAMECAD 10g-19mm XDrive				Working Sheet: Truss.1				Assembly Weight			38.1lbs

Powered by FRAMECAD Structure ® Minimum number of fasteners required is 2 per joint

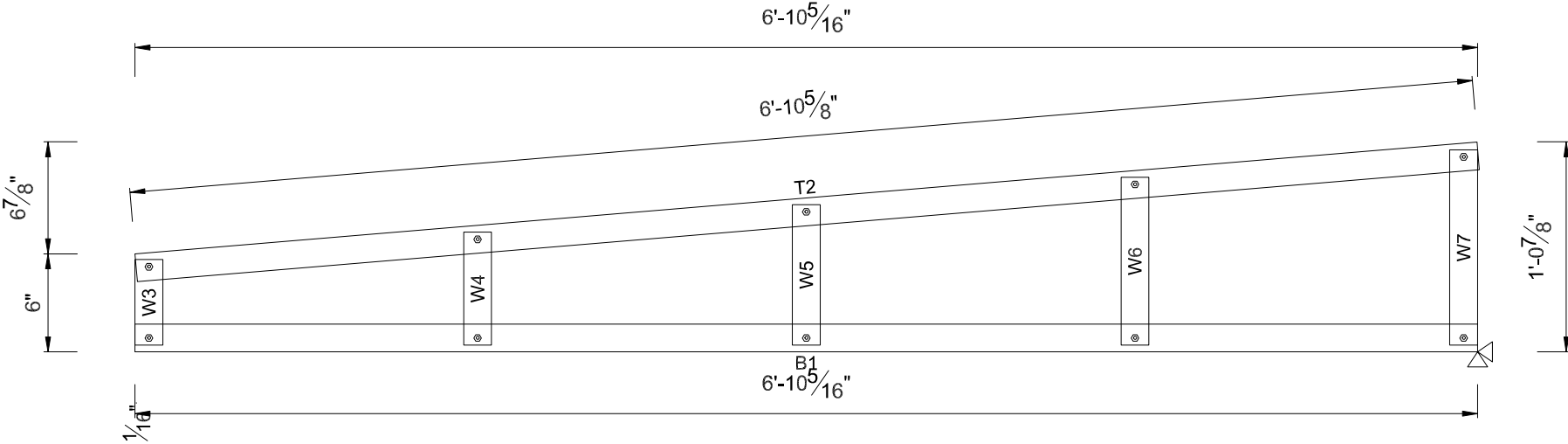


Quantity Required = 9 Mark as HN2 Engineering Status = 100%

System Name:	FRAMECAD_FT_i	Roof Type:	SHEET	Truss Model:	M11 362 LE System	Wind Speed:	W114
Truss Pitch:	4.7636	Truss Spacing:	2'-0"	Design Code:	AISI S100-12 LRFD	Loading Code:	IBC 2018 LRFD
Top Chord Live Load (psf):	20.0	Top Chord Dead Load (psf):	10.0	Bottom Chord Dead Load (psf):	5.0	Top Chord Snow Load (psf):	25.0
Envelope:	1'-4 5/8"h x 12'-6 1/8"w						

B1 362S162-33-50	1 6'-10 5/16"	T2 362S162-33-50	1 6'-10 9/16"	W3 362S162-33-50	1 5 1/4"	W4 362S162-33-50	1 6 15/16"
W5 362S162-33-50	1 8 9/16"	W6 362S162-33-50	1 10 1/4"	W7 362S162-33-50	1 11 15/16"	FRAMECAD 10g-19mm XDrive 20	
Working Sheet: Truss.1		Assembly Weight		16.1lbs			

Powered by FRAMECAD Structure [®]
 Minimum number of fasteners required is 2 per joint



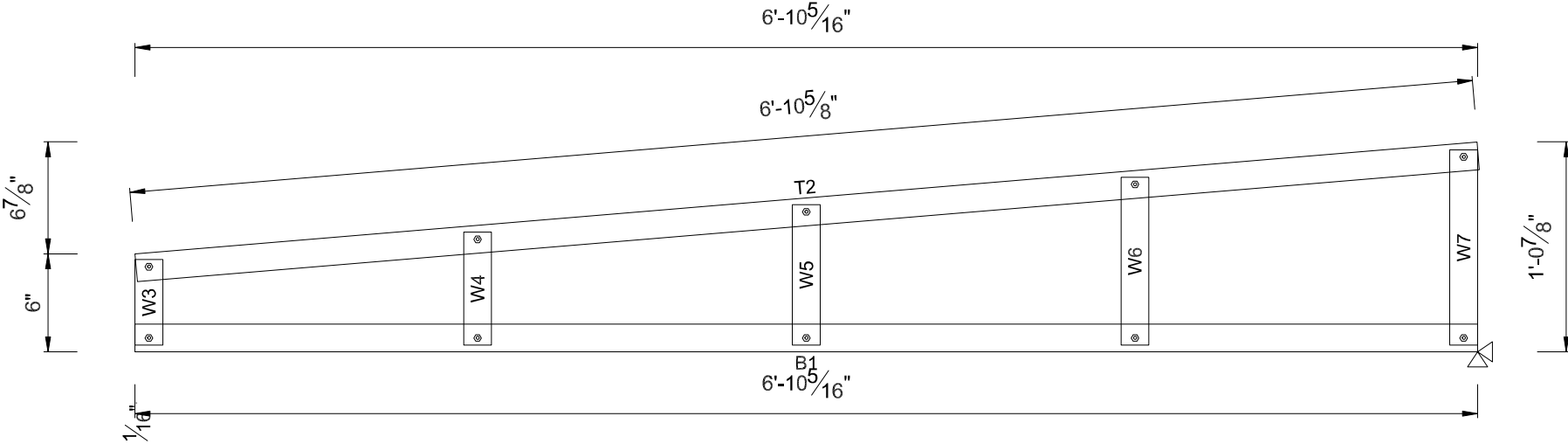
Quantity Required = 1 Mark as HG1 Engineering Status = Non Structural

System Name:	FRAMECAD_FT_i	Roof Type:	SHEET	Truss Model:	M11 362 LE System	Wind Speed:	W114
Truss Pitch:	4.764	Truss Spacing:	2'-0"	Envelope:	1'-7/8"h x 6'-10 7/16"w		

B1 362S162-33-50	1 6'-10 5/16"	T2 362S162-33-50	1 6'-10 9/16"	W3 362S162-33-50	1 5 1/4"	W4 362S162-33-50	1 6 15/16"
W5 362S162-33-50	1 8 9/16"	W6 362S162-33-50	1 10 1/4"	W7 362S162-33-50	1 11 15/16"	FRAMECAD 10g-19mm XDrive 20	
Working Sheet: Truss.1		Assembly Weight		16.1lbs			

Powered by FRAMECAD Structure ®

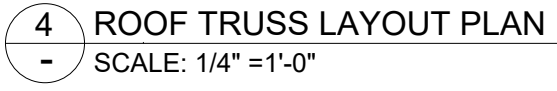
Minimum number of fasteners required is 2 per joint



Quantity Required = 1 Mark as HG2 Engineering Status = Non Structural

System Name:	FRAMECAD_FT_i	Roof Type:	SHEET	Truss Model:	M11 362 LE System	Wind Speed:	W114
Truss Pitch:	4.764	Truss Spacing:	2'-0"	Envelope:	1'-7/8"h x 6'-10 7/16"w		

SCOTSTEEL(2" HAT)			
Truss name	Weight(lbs)	Quantity	Total weight(lbs.)
E200	9.96	12	119.52
E201	9.22	2	18.44
HN1	28.63	2	57.26
HN2	28.63	9	257.67
HN3	91.83	7	642.81
GI1	294	1	251.35
HG1	28.63	2	57.26
HG2	134.4	1	91.83
Total Roof Weight (lbs.)			1496.14



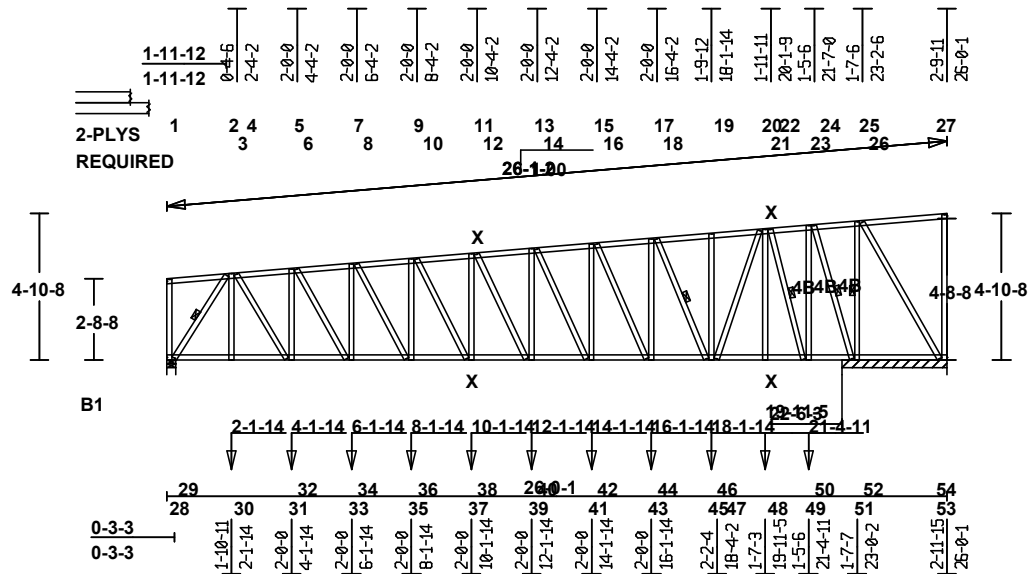
A. REF. TO ENGINEERED SHOP DRAWINGS FOR FASTENER & TRUSS DETAILS.

S-04

Job Name: Truss ID: G11 Qty: 1 SPACING: 2- 0- 0 PLY: 2 WEIGHT: 251.35

TC	FORCE	AXL	BND	CSI	ID	SCR/PLY
1-2	30	0.00	0.54	0.54	1	-SP
2-3	-1492	0.15	0.50	0.63	1	SP-
3-4	-1500	0.15	0.43	0.57	1	-SP
4-5	-2372	0.24	0.43	0.65	1	SP-SP
5-6	-2492	0.25	0.32	0.54	1	SP-
6-7	-3048	0.30	0.18	0.46	1	
7-8	-3111	0.31	0.18	0.47	1	
8-9	-3403	0.34	0.13	0.45	1	
9-10	-3427	0.34	0.13	0.45	1	
10-11	-3506	0.35	0.08	0.42	1	
11-12	-3491	0.35	0.08	0.42	1	
12-13	-3386	0.34	0.09	0.41	1	
13-14	-3333	0.33	0.13	0.44	1	
14-15	-3077	0.31	0.13	0.42	1	
15-16	-2991	0.30	0.21	0.48	1	
16-17	-2598	0.26	0.21	0.44	1	
17-18	-2460	0.25	0.20	0.43	1	
18-19	-1951	0.20	0.23	0.41	1	
19-20	-1976	0.20	0.68	0.86	1	-SP
20-21	-884	0.09	0.68	0.77	1	SP-
21-22	-836	0.08	0.80	0.88	1	-SP
22-23	-271	0.02	0.80	0.82	1	SP-SP
23-24	120	0.01	0.50	0.77	1	SP-SP
24-25	800	0.07	0.51	0.53	1	SP-SP
25-26	643	0.05	0.48	0.52	1	SP-
26-27	-24	0.00	0.31	0.31	1	

BC	FORCE	AXL	BND	CSI	ID	SCR/PLY
28-29	0	0.00	0.02	0.02	1	-SP
29-30	1350	0.11	0.05	0.16	1	SP-
30-31	1350	0.11	0.18	0.29	1	-SP
31-32	2371	0.20	0.27	0.46	1	SP-SP
32-33	2371	0.20	0.27	0.46	1	SP-
33-34	3030	0.25	0.16	0.41	1	
34-35	3030	0.25	0.13	0.37	1	
35-36	3382	0.28	0.09	0.36	1	
36-37	3382	0.28	0.09	0.36	1	
37-38	3481	0.29	0.02	0.31	1	
38-39	3481	0.29	0.04	0.32	1	
39-40	3357	0.28	0.04	0.31	1	
40-41	3357	0.28	0.10	0.38	1	
41-42	3046	0.25	0.10	0.35	1	
42-43	3046	0.25	0.09	0.33	1	
43-44	2562	0.21	0.22	0.42	1	
44-45	2562	0.21	0.53	0.72	1	
45-46	1928	0.16	0.53	0.67	1	
46-47	1928	0.16	0.49	0.64	1	-SP
47-48	1070	0.09	0.49	0.57	1	SP-
48-49	1070	0.09	0.61	0.68	1	-SP
49-50	232	0.01	0.61	0.86	1	SP-SP
50-51	232	0.01	0.50	0.50	1	SP-
51-52	0	0.00	0.05	0.06	1	SP-SP
52-53	0	0.00	0.12	0.12	1	SP-
53-54	0	0.00	0.12	0.12	1	



OVER SUPPORT AS SHOWN

Scale: 5/32" = 1'

NUTRUSSTM
A NUCONSTEEL Product

WARNING Read all notes on this sheet and verify all design parameters.
Truss design on this sheet is only valid with NUTRUSST sections and is for an individual building component, not a truss system. Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is part of the building design and which must be considered by the building designer. Bracing shown is lateral bracing of truss members only. Any additional bracing, temporary and/or permanent, is the responsibility of the truss erector and/or the building designer. The Professional Engineer's seal indicates only that the truss assembly shown on this sheet meets the acceptable design criteria for the loads, loading condition, truss configuration and spans specified.

Designer:		WO: SDP_Trusses
Dsgn Chk:		
Engg Chk:		
Cutting :		
TC Live	20.00 psf	Design Spec: AISI S100-2012
TC Dead	20.00 psf	Buildg Spec: IBC-2018
BC Live	0.00 psf	
BC Dead	0.00 psf	
TOTAL	40.00 psf	Date: 11/23/2022@ 17:47:58 Seqn S8.1.0a - 24864

NUTRUSSTM A NUCONSTEEL Product	WARNING Read all notes on this sheet and verify all design parameters. Truss design on this sheet is only valid with NUTRUSS sections and is for an individual building component, not a truss system. Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is part of the building design and which must be considered by the building designer. Bracing shown is lateral bracing of truss members only. Any additional bracing, temporary and/or permanent, is the responsibility of the truss erector and/or the building designer. The Professional Engineer's seal indicates only that the truss assembly shown on this sheet meets the acceptable design criteria for the loads, loading condition, truss configuration and spans specified.	Designer: Dsgn Chk: Engg Chk: Cutting :		WO: SDP_Trusses	
		TC Live	20.00 psf	Design Spec: AISI S100-2012	
		TC Dead	20.00 psf	Buildg Spec: IBC-2018	
		BC Live	0.00 psf		
		BC Dead	0.00 psf		
TOTAL	40.00 psf	Date: 11/23/2022@ Seqn S8.1.0a - 24863		17:47:	

Job Name: Truss ID: HG2 Qty: 1 SPACING: 2- 0- 0 PLY: 1 WEIGHT: 91.83

TC	FORCE	AXL	BND	CSI	ID	SCRWS
OL-1	13	0.00	0.52	0.52	1	
1-2	11	0.00	0.53	0.53	1	-SP
2-3	-614	0.12	0.31	0.44	1	SP-SP
3-4	-1030	0.20	0.43	0.61	1	SP-SP
4-5	-1108	0.22	0.43	0.63	1	SP-SP
5-6	-1442	0.28	0.38	0.65	1	
6-7	-1862	0.37	0.38	0.72	1	
7-8	-2034	0.40	0.29	0.70	1	
8-9	-2232	0.45	0.29	0.70	1	
9-10	-2240	0.45	0.19	0.65	1	
10-11	-2216	0.44	0.28	0.69	1	
11-12	-2057	0.41	0.28	0.70	1	
12-13	-1813	0.36	0.39	0.72	1	
13-14	-1485	0.30	0.39	0.67	1	
14-15	-1012	0.20	0.44	0.61	1	-SP
15-16	-512	0.10	0.43	0.52	1	SP-SP
16-17	-419	0.08	0.31	0.45	1	SP-SP
17-18	-11	0.00	0.52	0.52	1	SP-
18-OR	-13	0.00	0.53	0.53	1	

BC	FORCE	AXL	BND	CSI	ID	SCRWS
19-20	24	0.00	0.23	0.23	1	-SP
20-21	540	0.08	0.23	0.31	1	SP-SP
21-22	957	0.15	0.44	0.58	1	SP-SP
22-23	1046	0.17	0.44	0.59	1	SP-
23-24	1381	0.16	0.37	0.52	1	
24-25	1824	0.20	0.54	0.54	1	
25-26	1997	0.22	0.25	0.47	1	
26-27	2219	0.25	0.66	0.66	1	
27-28	2227	0.25	0.20	0.44	1	
28-29	2227	0.25	0.66	0.66	1	
29-30	2069	0.24	0.24	0.47	1	
30-31	1849	0.20	0.66	0.66	1	
31-32	1522	0.17	0.31	0.48	1	
32-33	1072	0.11	0.63	0.63	1	-SP
33-34	574	0.09	0.30	0.46	1	SP-SP
34-35	493	0.06	0.40	0.45	1	SP-SP
35-36	-23	0.00	0.24	0.24	1	SP-

WEB	FORCE	CSI	ID	SCRWS
1-19	-284	0.20	1	
2-20	-1145	0.37	1	SP-SP
3-21	1033	0.17	1	SP-SP
4-22	-1095	0.76	1	SP-SP
5-23	846	0.14	1	
6-24	-797	0.77	1	
7-25	387	0.06	1	
8-26	-403	0.39	1	
9-27	137	0.07	1	
10-28	137	0.10	1	
11-29	-355	0.30	1	
12-30	399	0.07	1	
13-31	-729	0.62	1	
14-32	813	0.13	1	
15-33	-1118	0.94	1	SP-SP
16-34	973	0.16	1	SP-SP
17-35	-1083	0.86	1	SP-SP
18-36	-282	0.19	1	

UPLIFT REACTION(S) :
Support C&C Wind Main Wind Non-Wind
1 -38 lb
2 -107 lb

Type ID SECTION Fy(ksi) Joints
TC 1 20TC20 50
BC 1 20TC20 50
WEB 1 20TC20 50

This truss is designed using the
ASCE7-16 Wind Specification
Bldg Enclosed = Yes,
Truss Location = Not End Zone
Exp Category = C
Bldg Length = 26.00 ft, Bldg Width = 26.00 ft
Mean roof height = 11.72 ft, mph = 96
Occupancy Category II, Wind Dead Load = 12.00 psf
Designed as Main Wind Force Resisting System
- Low-rise and Components and Cladding
Tributary Area = 52 sqft
Uplifts based on elevation at or above 0 ft

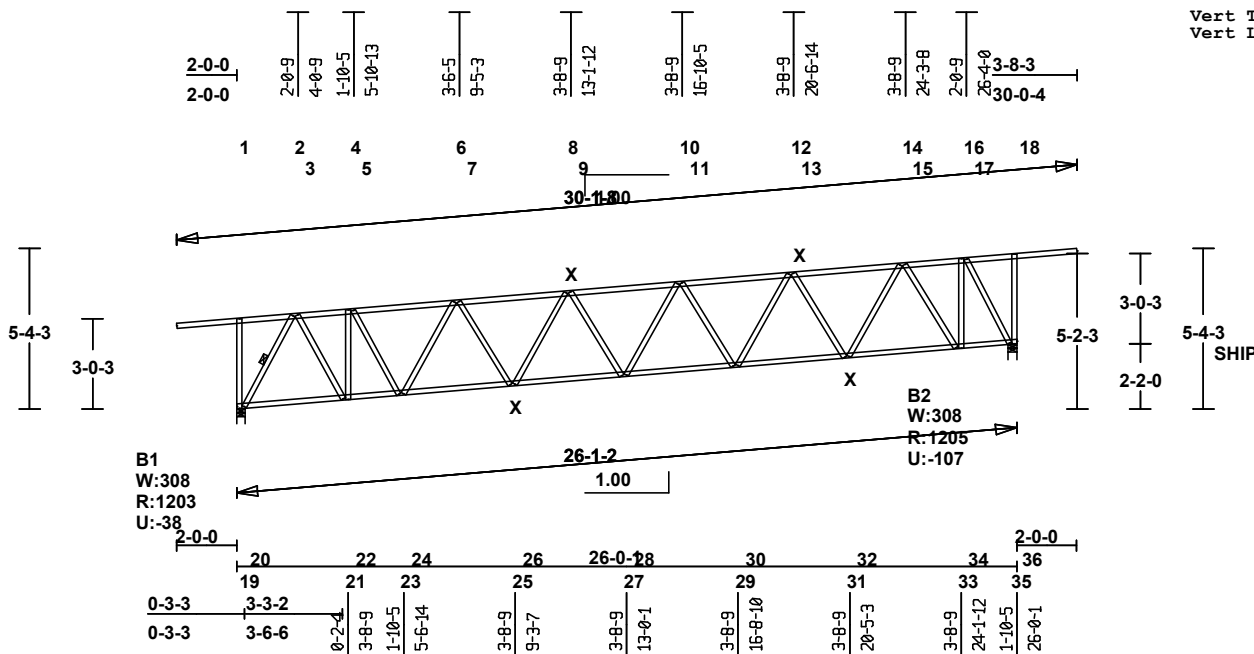
THIS DESIGN IS THE COMPOSITE RESULT OF
MULTIPLE LOAD CASES.
Loaded for 10 PSF non-concurrent BCLL.
Loaded for 200 lb non-concurrent moving
BCLL.
Shim bearings (if needed) for req. support.
End verticals are designed for axial loads
only unless noted otherwise.
Extensions above or below the truss profile
(if any) have been designed for loads
indicated only. Horizontal loads applied at
the end of the extensions have not been
considered unless shown. A drop-leg to an
otherwise unsupported wall may create a
hinge effect that requires additional design
consideration (by others).
20 psf bottom chord live load NOT required
on this truss, per IBC/IRC requirements for
attics with limited storage.

This design based on chord bracing applied
per the following schedule:
max o.c. from to
TC 12.00" -2- 0- 0 28- 0- 1
BC 12.00" 0- 0- 0 26- 0- 1
Galvanization: G60

BRG	X-LOC	SIZE	REACT	HORIZ
1	0- 1-12	3.50"	1203	67
2	25-10- 5	3.50"	1205	67

	DEFLECTION	LOC.	ALLOW.	LC
Vert TL:	-0.27" (L/999)	9-10	L/240	83
Vert LL:	-0.14" (L/999)	9-10	L/360	83
Horz TL:	0.07"			
Cantilever				
Vert TL:	-0.15" (L/186)	OL-1	L/ 90	79
Vert LL:	-0.09" (L/317)	OL-1	L/120	79

===== Joint Locations =====				
1	0- 0- 0	19	0- 0- 0	
2	1-10- 4	20	0- 3- 3	
3	2- 0- 9	21	3- 6- 6	
4	3- 8- 9	22	3- 8- 9	
5	3-10-13	23	5- 4- 9	
6	7- 2-14	24	5- 6-14	
7	7- 5- 3	25	9- 1- 3	
8	10-11- 7	26	9- 3- 7	
9	11- 1-12	27	12- 9-12	
10	14- 8- 1	28	13- 0- 1	
11	14-10- 5	29	16- 6- 5	
12	18- 4-10	30	16- 8-10	
13	18- 6-14	31	20- 2-15	
14	22- 1- 3	32	20- 5- 3	
15	22- 3- 8	33	23-11- 8	
16	24- 1-12	34	24- 1-12	
17	24- 4- 1	35	25- 8-13	
18	26- 0- 1	36	26- 0- 1	
== X-Brac. Locations (Joints) ==				
BC	TC			
25	8			
31	13			



Each connection requires 3/8" diameter proprietary bolt supplied by NUCONSTEEL
SCRWS = The required number of double-sided #14 screws at each end of the truss member: SP = Spacer supplied by NUCONSTEEL
This truss has no Pre-Camber

Scale: 5/32" = 1'

NUTRUSSTM
A NUCONSTEEL Product

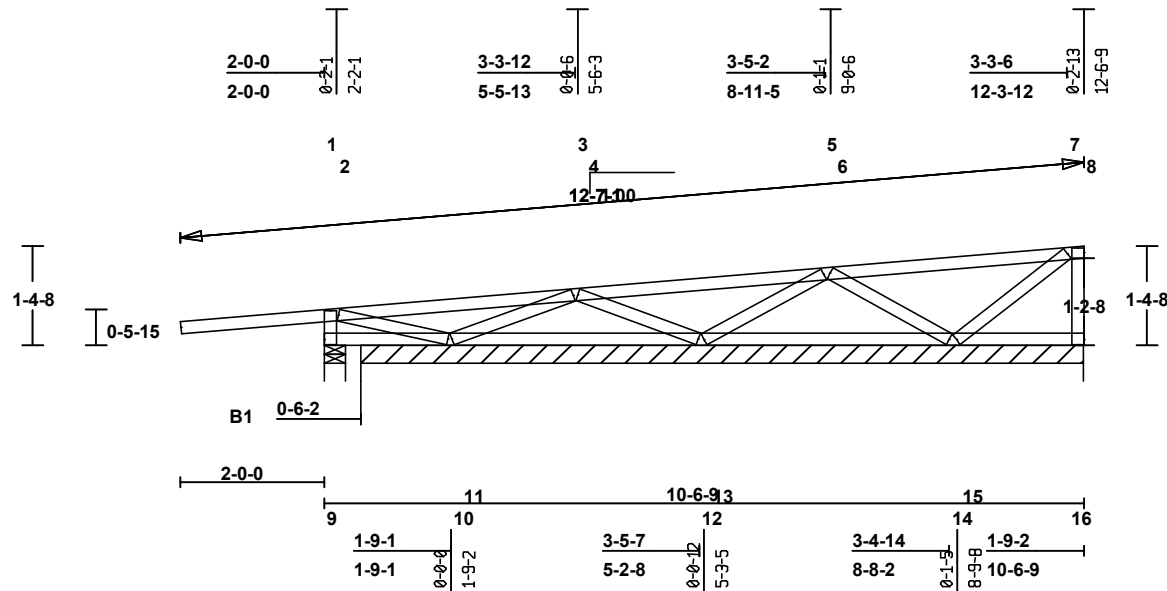
WARNING Read all notes on this sheet and verify all design parameters.
Truss design on this sheet is only valid with NUTRUSST sections and is for an individual building component, not a truss system. Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is part of the building design and which must be considered by the building designer. Bracing shown is lateral bracing of truss members only. Any additional bracing, temporary and/or permanent, is the responsibility of the truss erector and/or the building designer. The Professional Engineer's seal indicates only that the truss assembly shown on this sheet meets the acceptable design criteria for the loads, loading condition, truss configuration and spans specified.

Designer: Dsgn Chk: Engg Chk: Cutting :	WO: SDP_Trusses
TC Live 20.00 psf TC Dead 20.00 psf BC Live 0.00 psf BC Dead 0.00 psf TOTAL 40.00 psf	Design Spec: AISI S100-2012 Buldg Spec: IBC-2018 Date: 11/23/2022@ Seqn S8.1.0a - 24863

17:47:03

Job Name: Truss ID: HG1 Qty: 2 SPACING: 2- 0-0 PLY: 1 WEIGHT: 28.63

TC FORCE AXL BND CSI ID SCRWS										UPLIFT REACTION(S) :				NOTE:MAXIMUM TRUSS SPACING IS 20.7 " O.C.				This design based on chord bracing applied per the following schedule:			
OL-1 17 0.00 0.67 0.67 1										Support C&C Wind Main Wind Non-Wind				THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.				max o.c. from to			
1-2 -22 0.00 0.62 0.62 1										2 -48 lb				Loaded for 10 PSF non-concurrent BCLL.				TC 12.00" -2- 0- 0 10- 6- 9			
2-3 -279 0.06 0.73 0.78 1										3 -22 lb				Loaded for 200 lb non-concurrent moving BCLL.				BC 12.00" 0- 0- 0 10- 6- 9			
3-4 -461 0.09 0.30 0.38 1										4 -13 lb				Mark all interior bearing locations.				Galvanization: G60			
4-5 98 0.01 0.34 0.34 1										5 -4 lb				Install interior support(s) before erection.							
5-6 -152 0.03 0.37 0.40 1										6 -7 lb -5 lb -37 lb				This truss is designed using the							
6-7 349 0.05 0.37 0.41 1										7 -11 lb				ASCE7-16 Wind Specification							
7-8 -8 0.00 0.06 0.06 1										8 -32 lb -23 lb -62 lb				Bldg Enclosed = Yes,							
										Type ID SECTION Fy(ksi) Joints				Truss Location = Not End Zone							
BC FORCE AXL BND CSI ID SCRWS										TC 1 20TC20 50				Exp Category = C							
9-10 0 0.00 0.09 0.09 1										BC 1 20TC20 50				Bldg Length = 26.00 ft, Bldg Width = 26.00 ft							
10-11 -173 0.03 0.01 0.15 1										WEB 1 20TC20 50				Mean roof height = 9.65 ft, mph = 100							
11-12 0 0.00 0.11 0.11 1														Occupancy Category II, Wind Dead Load = 18.00 psf							
12-13 -185 0.04 0.06 0.10 1														Designed as Main Wind Force Resisting System							
13-14 16 0.00 0.13 0.13 1														- Low-rise and Components and Cladding							
14-15 -316 0.02 0.13 0.15 1														Tributary Area = 18 sqft							
15-16 20 0.00 0.18 0.18 1														Uplifts based on elevation at or above 0 ft							
WEB FORCE CSI ID SCRWS																					
1-9 -472 0.09 1																					
2-10 274 0.05 1																					
3-11 -479 0.16 1																					
4-12 -549 0.17 1																					
5-13 -238 0.08 1																					
6-14 -551 0.18 1																					
7-15 -426 0.15 1																					
8-16 70 0.01 1																					



		DEFLECTION	LOC.	ALLOW.	LC
Vert TL:	-0.03"	(L/999)	6-7	L/240	62
Vert LL:	-0.02"	(L/999)	6-7	L/360	62
Horz TL:	0.00"				
Cantilever					
Vert TL:	-0.21"	(L/114)	OL-1	L/ 90	63
Vert LL:	-0.13"	(L/187)	OL-1	L/120	63

===== Joint Locations =====					
1	0- 0- 0	9	0- 0- 0		
2	0- 2- 1	10	1- 9- 1		
3	3- 5-13	11	1- 9- 2		
4	3- 6- 3	12	5- 2- 8		
5	6-11- 5	13	5- 3- 5		
6	7- 0- 6	14	8- 8- 2		
7	10- 3-12	15	8- 9- 8		
8	10- 6- 9	16	10- 6- 9		

Each connection requires 3/8" diameter proprietary bolt supplied by NUCONSTEEL OVER SUPPORT AS SHOWN
SCRWS = The required number of double-sided #14 screws at each end of the truss member: SP = Spacer supplied by NUCONSTEEL
Scale: 3/8" = 1'

NUTRUSSTMTA NUCONSTEEL Product

WARNING

Read all notes on this sheet and verify all design parameters.

Truss design on this sheet is only valid with NUTRUSST sections and is for an individual building component, not a truss system. Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is part of the building design and which must be considered by the building designer. Bracing shown is lateral bracing of truss members only. Any additional bracing, temporary and/or permanent, is the responsibility of the truss erector and/or the building designer. The Professional Engineer's seal indicates only that the truss assembly shown on this sheet meets the acceptable design criteria for the loads, loading condition, truss configuration and spans specified.

Designer:

Dsgn Chk:

Engg Chk:

Cutting :

TC Live30.00 psf

TC Dead30.00 psf

BC Live0.00 psf

BC Dead0.00 psf

TOTAL60.00 psf

WO: SDP_TRUSSES

Design Spec: AISI S100-2012

Buildg Spec: IBC-2018

Date: 11/17/2022@19:29:03

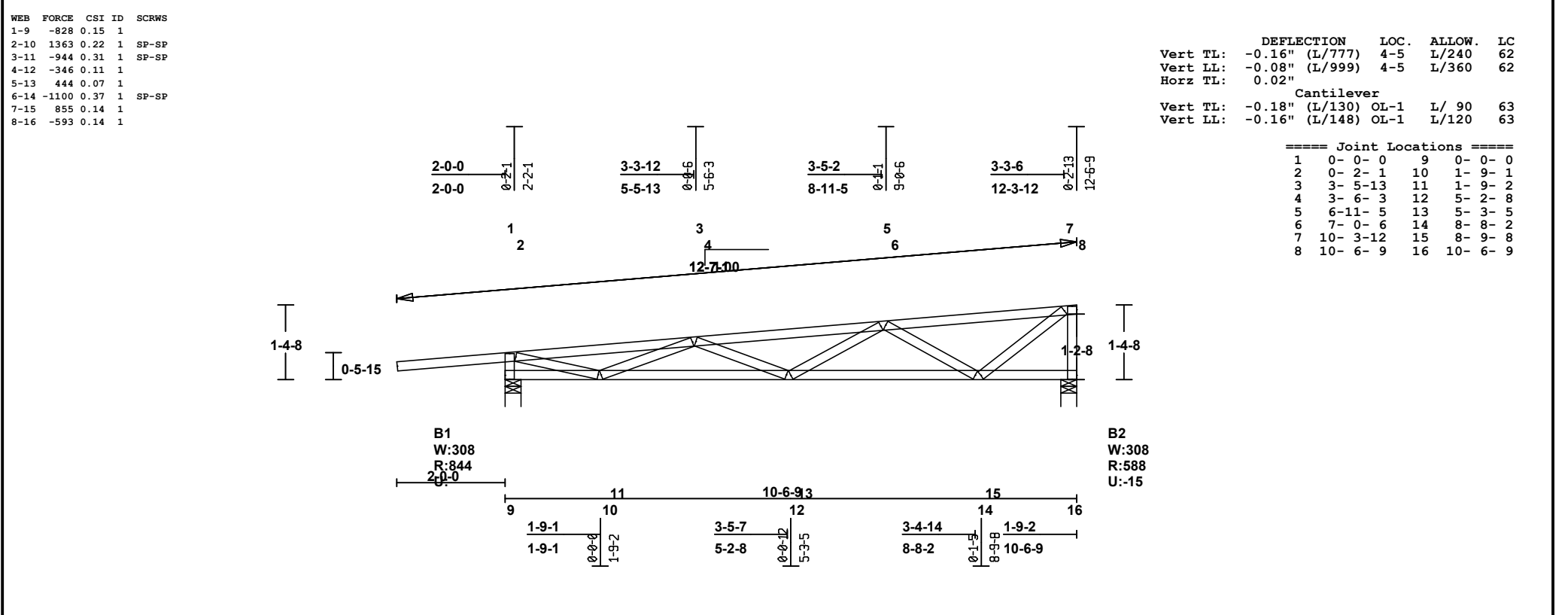
Seqn S8.1.0a - 24641

Job Name: Truss ID: HN2 Qty: 11 SPACING: 2-0-0 PLY: 1 WEIGHT: 28.63

TC FORCE AXL BND CSI ID SCRWS										UPLIFT REACTION(S) :				NOTE:MAXIMUM TRUSS SPACING IS 22.7 " O.C.				This design based on chord bracing applied per the following schedule:			
OL-1 19 0.00 0.73 0.73 1										Support C&C Wind Main Wind Non-Wind				THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.				max o.c. from to			
1-2 -50 0.01 0.68 0.69 1 -SP										2 -15 lb				Loaded for 10 PSF non-concurrent BCLL.				TC 12.00" -2- 0- 0 10- 6- 9			
2-3 -1351 0.11 0.62 0.72 1 SP-SP										Type ID SECTION Fy(ksi) Joints				Loaded for 200 lb non-concurrent moving BCLL.				BC 12.00" 0- 0- 0 10- 6- 9			
3-4 -2231 0.45 0.24 0.65 1 SP-										TC 1 20TC20 50				Galvanization: G60							
4-5 -1917 0.38 0.38 0.76 1										BC 1 20TC20 50				This truss is designed using the							
5-6 -1615 0.32 0.49 0.78 1 -SP										WEB 1 20TC20 50				ASCE7-16 Wind Specification							
6-7 -700 0.14 0.54 0.67 1 SP-										20 psf bottom chord live load NOT required on this truss, per IBC/IRC requirements for attics with limited storage.				Bldg Enclosed = Yes,							
7-8 49 0.01 0.42 0.42 1														Truss Location = Not End Zone							
														Exp Category = C							
														Bldg Length = 26.00 ft, Bldg Width = 26.00 ft							
														Mean roof height = 9.65 ft, mph = 100							
														Occupancy Category II, Wind Dead Load = 18.00 psf							
														Designed as Main Wind Force Resisting System							
														- Low-rise and Components and Cladding							
														Tributary Area = 20 sqft							
														Uplifts based on elevation at or above 0 ft							

Job Name: Truss ID: HN1 Qty: 2 SPACING: 2-0-0 PLY: 1 WEIGHT: 28.63

TC FORCE AXL BND CSI ID SCRWS										UPLIFT REACTION(S) :			NOTE:MAXIMUM TRUSS SPACING IS 22.7 " O.C.			This design based on chord bracing applied per the following schedule:		
OL-1 19 0.00 0.73 0.73 1										Support C&C Wind Main Wind Non-Wind			THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.			max o.c. from to		
1-2 -50 0.01 0.68 0.69 1 -SP										2 -15 lb			Loaded for 10 PSF non-concurrent BCLL.			TC 12.00" -2- 0- 0 10- 6- 9		
2-3 -1351 0.11 0.62 0.72 1 SP-SP										Type ID SECTION Fy(ksi) Joints			Loaded for 200 lb non-concurrent moving BCLL.			BC 12.00" 0- 0- 0 10- 6- 9		
3-4 -2231 0.45 0.24 0.65 1 SP-										TC 1 20TC20 50						Galvanization: G60		
4-5 -1917 0.38 0.38 0.76 1										BC 1 20TC20 50						This truss is designed using the		
5-6 -1615 0.32 0.49 0.78 1 -SP										WEB 1 20TC20 50						ASCE7-16 Wind Specification		
6-7 -700 0.14 0.54 0.67 1 SP-										20 psf bottom chord live load NOT required on this truss, per IBC/IRC requirements for attics with limited storage.			BRG X-LOC SIZE REACT HORIZ			Bldg Enclosed = Yes,		
7-8 49 0.01 0.42 0.42 1													1 0- 1-12 3.50" 844 34			Truss Location = Not End Zone		
BC FORCE AXL BND CSI ID SCRWS													2 10- 4-13 3.50" 588 34			Exp Category = C		
9-10 -34 0.00 0.18 0.18 1 -SP																Bldg Length = 26.00 ft, Bldg Width = 26.00 ft		
10-11 -310 0.06 0.14 0.52 1 SP-SP																Mean roof height = 9.65 ft, mph = 100		
11-12 2221 0.26 0.68 0.68 1 SP-																Occupancy Category II, Wind Dead Load = 18.00 psf		
12-13 1896 0.23 0.14 0.37 1																Designed as Main Wind Force Resisting System		
13-14 1636 0.20 0.65 0.65 1 -SP																- Low-rise and Components and Cladding		
14-15 675 0.10 0.23 0.32 1 SP-																Tributary Area = 20 sqft		
15-16 34 0.00 0.23 0.23 1																Uplifts based on elevation at or above 0 ft		

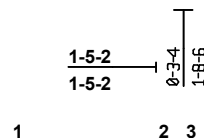


Each connection requires 3/8" diameter proprietary bolt supplied by NUCONSTEEL
SCRWS = The required number of double-sided #14 screws at each end of the truss member: SP = Spacer supplied by NUCONSTEEL

NUTRUSSTM A NUCONSTEEL Product	WARNING Read all notes on this sheet and verify all design parameters. Truss design on this sheet is only valid with NUTRUSST sections and is for an individual building component, not a truss system. Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is part of the building design and which must be considered by the building designer. Bracing shown is lateral bracing of truss members only. Any additional bracing, temporary and/or permanent, is the responsibility of the truss erector and/or the building designer. The Professional Engineer's seal indicates only that the truss assembly shown on this sheet meets the acceptable design criteria for the loads, loading condition, truss configuration and spans specified.		Scale: 3/8" = 1'	
	Designer: Dsgn Chk: Engg Chk: Cutting :		WO: SDP_TRUSSES	
	TC Live 30.00 psf TC Dead 30.00 psf BC Live 0.00 psf BC Dead 0.00 psf TOTAL 60.00 psf		Design Spec: AISI S100-2012 Bldg Spec: IBC-2018 Date: 11/17/2022@ Seqn S8.1.0a - 24640	

Job Name:	Truss ID: E200	Qty: 12	SPACING: 2- 0- 0	PLY: 1	WEIGHT: 9.96
------------------	-----------------------	----------------	-------------------------	---------------	---------------------

TC	FORCE	AXL	BND	CSI	ID	SCRWS	UPLIFT REACTION(S) :	THIS DESIGN IS THE COMPOSITE RESULT OF	This design based on chord bracing applied
1-2	0	0.00	0.09	0.09	1	Support	C&C Wind Main Wind Non-Wind	MULTIPLE LOAD CASES.	per the following schedule:
2-3	0	0.00	0.05	0.05	1	2	-26 lb -1 lb	Loaded for 10 PSF non-concurrent BCLL.	max o.c. from to
						3	-18 lb -1 lb	Loaded for 200 lb non-concurrent moving	TC 12.00" 0- 0- 0 1- 8- 6
BC	FORCE	AXL	BND	CSI	ID	SCRWS	Type ID SECTION Fy(ksi) Joints	BCLL.	BC 12.00" 0- 0- 0 1- 8- 6
4-5	0	0.00	0.06	0.06	1	TC 1	20TC20 50	Mark all interior bearing locations.	Galvanization: G60
5-6	0	0.00	0.07	0.07	1	BC 1	20TC20 50	Install interior support(s) before erection.	
						WEB 1	20TC20 50	Drainage must be provided to avoid ponding.	
WEB	FORCE	CSI	ID	SCRWS			This truss is designed using the	REACTIONS	
1-4	-68	0.05	1				ASCE7-16 Wind Specification	BRG X-LOC SIZE REACT HORIZ	
2-5	0	0.00	1				Bldg Enclosed = Yes,	1 0- 1-12 3.50" 0 0	
3-6	-68	0.05	1				Truss Location = Not End Zone	2 0- 3- 4 3.50" 208 0	
							Exp Category = C	3 1- 6-10 3.50" 133 0	
							Bldg Length = 26.00 ft, Bldg Width = 26.00 ft		
							Mean roof height = 12.80 ft, mph = 96		
							Occupancy Category II, Wind Dead Load = 12.00 psf		
							Designed as Main Wind Force Resisting System		
							- Low-rise and Components and Cladding		
							Tributary Area = 3 sqft		
							Uplifts based on elevation at or above 0 ft		
							on this truss, per IBC/IRC requirements for		
							attics with limited storage.		

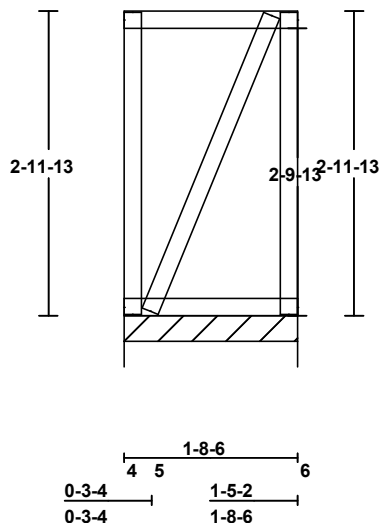


	DEFLECTION	LOC.	ALLOW.	LC
Vert TL:	0.00" (L/999)	1-2	L/240	48
Vert LL:	0.00" (L/999)	1-2	L/360	48
Horz TL:	0.01"			

```

===== Joint Locations =====
 1  0- 0- 0      4  0- 0- 0
 2  1- 5- 2      5  0- 3- 4
 3  1- 8- 6      6  1- 8- 6

```



Each connection requires 3/8" diameter proprietary bolt supplied by NUCONSTEEL
 SCRWS = The required number of double-sided #14 screws at each end of the truss member: SP = Spacer supplied by NUCONSTEEL
 This truss has no Pre-Camber

OVER CONTINUOUS SUPPORT
 Scale: 17/32" = 1'

NUTRUSS™ A NUCONSTEEL Product	WARNING <i>Read all notes on this sheet and verify all design parameters.</i> Truss design on this sheet is only valid with NUTRUSS sections and is for an individual building component, not a truss system. Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is part of the building design and which must be considered by the building designer. Bracing shown is lateral bracing of truss members only. Any additional bracing, temporary and/or permanent, is the responsibility of the truss erector and/or the building designer. The Professional Engineer's seal indicates only that the truss assembly shown on this sheet meets the acceptable design criteria for the loads, loading condition, truss configuration and spans specified.		Designer: Dsgn Chk: Engg Chk: Cutting :		WO: SDP_Trusses	
	TC Live	20.00 psf	Design Spec: AISI S100-2012			
	TC Dead	20.00 psf	Buildg Spec: IBC-2018			
	BC Live	0.00 psf				
	BC Dead	0.00 psf				
TOTAL	40.00 psf			Date: 11/23/2022@ 18:09: Seqn S8.1.0a - 24869		

TC	FORCE	AXL	BND	CSI	ID	SCRWS
1-2	0	0.00	0.05	0.05	1	
2-3	0	0.00	0.03	0.03	1	

BC	FORCE	AXL	BND	CSI	ID	SCRWS
4-5	0	0.00	0.04	0.05	1	
5-6	0	0.00	0.04	0.06	1	

WEB	FORCE	CSI	ID	SCRWS
1-4	-52	0.03	1	
2-5	0	0.00	1	
3-6	-52	0.03	1	

UPLIFT REACTION(S) :
Support C&C Wind Main Wind Non-Wind
2 -21 lb -1 lb
3 -12 lb

Type	ID	SECTION	Fy(ksi)	Joints
TC	1	20TC20	50	
BC	1	20TC20	50	
WEB	1	20TC20	50	

This truss is designed using the
ASCE7-16 Wind Specification
Bldg Enclosed = Yes,
Truss Location = Not End Zone
Exp Category = C
Bldg Length = 26.00 ft, Bldg Width = 26.00 ft
Mean roof height = 12.80 ft, mph = 96
Occupancy Category II, Wind Dead Load = 12.00 psf
Designed as Main Wind Force Resisting System
- Low-rise and Components and Cladding
Tributary Area = 3 sqft
Uplifts based on elevation at or above 0 ft

THIS DESIGN IS THE COMPOSITE RESULT OF
MULTIPLE LOAD CASES.
Loaded for 10 PSF non-concurrent BCLL.
Loaded for 200 lb non-concurrent moving
BCLL.
Mark all interior bearing locations.
Install interior support(s) before erection.
Drainage must be provided to avoid ponding.
End verticals are designed for axial loads
only unless noted otherwise.
Extensions above or below the truss profile
(if any) have been designed for loads
indicated only. Horizontal loads applied at
the end of the extensions have not been
considered unless shown. A drop-leg to an
otherwise unsupported wall may create a
hinge effect that requires additional design
consideration (by others).
20 psf bottom chord live load NOT required
for this truss, per IBC/IRC requirements for
attics with limited storage.

This design based on chord bracing applied
per the following schedule:

	max o.c.	from	to
TC	12.00"	0- 0- 0	1- 3- 8
BC	12.00"	0- 0- 0	1- 3- 8

Galvanization: G60

BRG	X-LOC	SIZE	REACT	HORIZ
1	0- 1-12	3.50"	0	0
2	0- 3- 4	3.50"	188	0
3	1- 1-12	3.50"	69	0

	DEFLECTION	LOC.	ALLOW.	LC
Vert TL:	0.00" (L/999)	1-2	L/240	48
Vert LL:	0.00" (L/999)	1-2	L/360	48
Horz TL:	0.00"			

=====	Joint Locations	=====
1	0- 0- 0	4 0- 0- 0
2	1- 0- 4	5 0- 3- 4
3	1- 3- 8	6 1- 3- 8

1 2 3

Each connection requires 3/8" diameter proprietary bolt supplied by NUCONSTEEL
SCRWS = The required number of double-sided #14 screws at each end of the truss member: SP = Spacer supplied by NUCONSTEEL
This truss has no Pre-Camber

OVER SUPPORT AS SHOWN

Scale: 17/32" = 1'